

***STRUCTURAL DESIGN - II***  
***Design of Water Tanks***  
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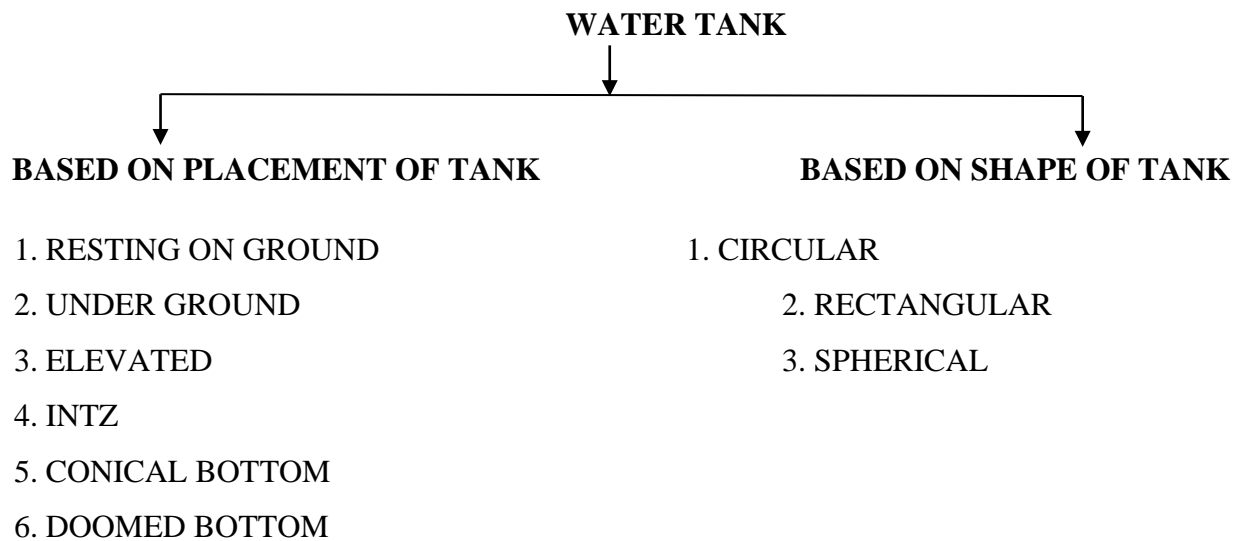
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# – Water Tanks –

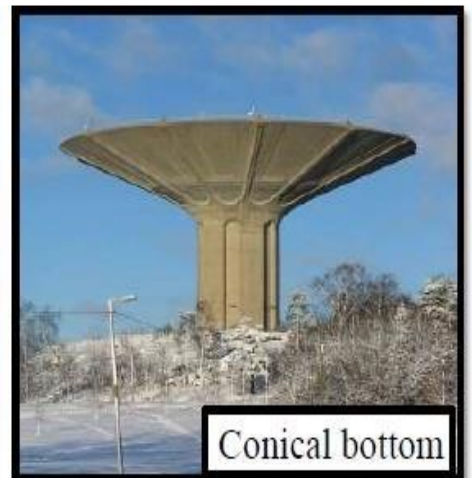
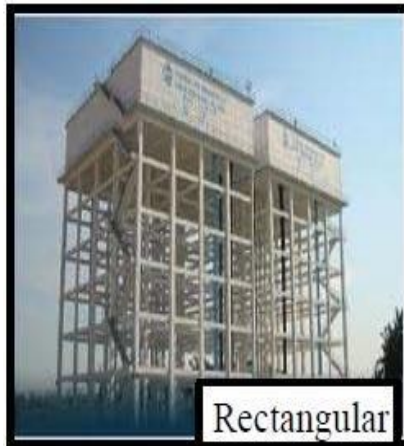
## WATER TANKS

### Introduction

A reinforced concrete tank is a very useful structure that is meant for the storage of water, swimming, and other purposes. Water tanks are used in uncracked theory where concrete is not allowed to crack on the tension side which means tensile stress is kept within permissible tensile stress in concrete.

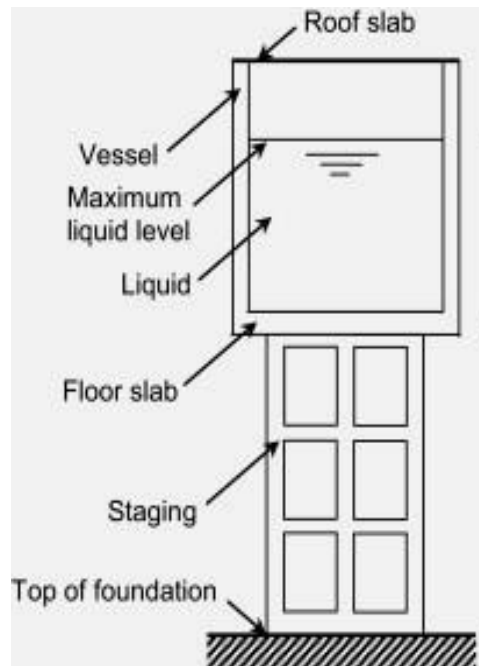


**Types of tanks based on shape**



Rectangular Tank	Circular Tank	Intze Tank	Prestressed Tanks
For smaller capacities rectangular tanks are adopted	For bigger capacities circular tanks are recommended	Intze tank is constructed to reduce the project cost because lower dome in this construction resists horizontal thrust.	RCC tanks are cheaper only for smaller capacities up to 10-12 lakhs liters. For bigger tanks, prestressing is the superior choice resulting in a saving of up to 20%.

**STAGING** - It is required for tanks above ground level. It consists of column, braces and foundation. It is designed for gravity load due to water and self weight, wind load and earthquake load.



*Stresses in concrete*

Grade of Concrete	Permissible Stress in N/mm <sup>2</sup>		Shear stress in N/mm <sup>2</sup>
	Direct tension	Tension due to bending	
M15	1.1	1.5	1.5
M20	1.2	1.7	1.7
M25	1.3	1.8	1.9
M30	1.5	2.0	2.2
M35	1.6	2.2	2.5
M40	1.7	2.4	2.7

*Permissible Stress*

Sl. No.	TYPES OF STRESS	PERMISSIBLE STRESS in N/mm <sup>2</sup>	
		Fe250	Fe415
1	Tensile stress in member under direct tension	115	150

2	Tensile stress in member in bending		
	a) on liquid retaining face of member	115	150
	b) on face away from liquid for member less than 225mm thick	115	150
	c) on face away from liquid for member greater than 225mm thick	125	190

Minimum area of steel = 0.3% gross area

Cover = 25mm

***ELEMENTS IN WATER TANK DESIGN:***

1. Roof slab
2. Wall
3. Base slab

***NOTE: walls and base slab comes under JOINTS which are of two types – RIGID and FLEXIBLE***

**THICKNESS OF WALL** - Should not be less than

- ❖ 150 mm
- ❖ 30mm per metre depth +50mm
- ❖ Thickness required limiting the tensile stress in concrete to  $1.3\text{N/mm}^2$  ( $M_{20}$ )

Circular tanks - Circular tanks on ground may be designed either with flexible connection of wall with base or with the rigid connection of wall with base in flexible connection the walls are not monolithic with base whereas in rigid connection walls are monolithic with base.

***Circular tank with flexible joint between wall and base***

The wall of such a tank will be designed as vertical cylinder subjected to water pressure. The intensity of water pressure at any depth  $h$ ,  $p = wh$  units per unit area. The corresponding hoop tension per meter height,

$$T = (Wh * D) / 2$$

Where ,

W= specific wt of water h=depth

D=diameter of tank

**Problem (1)** Design a circular tank resting on firm ground to the following particulars diameter of tank 3.5m, depth of water 3m, the wall and the base slab are not monolithic with each other. Specific weight of water is 9810 N/m<sup>3</sup>. Use M<sub>25</sub> grade concrete and Fe 415 steel bars

Step1: Given data

Diameter of tank=3.5m

Depth of water=3m

Specific wt of water =9810N/m<sup>3</sup>

M<sub>25</sub>

Step2: Thickness of wall

Should not be less than 150mm

30mm per meter depth + 50mm = (30\*3)+50 =140mm

Provide a thickness of 150mm

Step3: Reinforcements

Consider the bottom 1m height of wall

Pressure intensity corresponding to centre of bottom 1m height of wall

$$P = Wh$$

$$= 9810 * 2.5$$

$$= 24525 \text{ N/m}^2$$

$$\text{Hoop tension /m, } T = PD/2$$

$$= (24525 * 3.5) / 2$$

$$= 42919 \text{ N}$$

Steel required for 1m height ( $A_{st}$ ) =  $T / (\text{safe stress in steel})$  For Fe415 steel under direct tension, safe stress in steel = 150N/mm<sup>2</sup>

$$A_{st} = 42919/150$$

$$= 237\text{mm}^2$$

Min steel required ( $A_{st \text{ min}}$ ) = 0.3% gross area (gross area= $b \cdot T$ )

$$= (0.3/100) * 1000 * 150$$

$$= 450\text{mm}^2$$

Provide 10mm dia bars;

Spacing of 10mm dia bars =  $(1000 * 3.14 * (10^2/4))/450$

$$= 174\text{mm}$$

Provide 10mm bars @ 170 mm c/c spacing

Vertical distribution steel = 0.3 % gross

$$= 450\text{mm}^2$$

Step4: Base slab

Provide 150mm thick slab with top and bottom mesh with 10mm dia bars @250 mm c/c

Circular tanks with a wall retained at base (rigid) condition: in this case the wall will resist water pressure partly hoop action and partly by cantilever action at a certain height from the bottom there will be cantilever action and at a higher level hoop action

### ***Methods to analyse***

1. Dr. Reissners method
2. Carpenter's simplification of Dr. Reissners method
3. IS code

### **IS CODE**

$$H^2/(D \cdot t)$$

Hoop tension per metre height = coefficient \*  $WH/2$  N

BM per metre run = coefficient \*  $WH^3$  N.m

Shear force at base of wall = coefficient \*  $WH^2$  N

Problem (2) Design a circular tank 12 m diameter of 4m height. The tank rest on firm ground and the walls of the tanks are retained (rigid) use M<sub>20</sub> grade concrete and Fe 415 steel?

Solution:

Step1: given data

Diameter of tank = 12m

Height of tank = 4m

Walls are retained at base

Step2: Thickness of wall

Should not be less than 150mm

30mm per metre depth + 50mm =  $(30 \times 4) + 50 = 170$ mm provide

Step3: reinforcement

Hoop tension (refer under 0.6 H)

$$\begin{aligned} H^2/(D \cdot t) &= 4^2/(2 \cdot 0.17) \\ &= 7.81 \end{aligned}$$

Coefficient under 0.6H

6	0.514	7.8	?	8
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0.575

Interpolate:

$$(0.514 - (0.514 - 0.575) \cdot (7.8 - 6)) / (8 - 6)$$

Coefficient for 7.8 = 0.57

$$\begin{aligned} \text{Max hoop tension} &= \text{coefficient} \cdot (W \cdot H \cdot D) / 2 \\ &= (0.57 \cdot 9810 \cdot 4 \cdot 12) / 2 \\ &= 134.2 \cdot 10^3 \text{ N} \end{aligned}$$

$$\begin{aligned} A_{st} &= T / \text{safe stress in steel} \\ &= 134.2 \cdot 10^3 / 150 \\ &= 394.6 \text{ mm}^2 \end{aligned}$$

$$\begin{aligned} A_{st \text{ min}} &= 0.3\% \text{ gross area} \\ &= (0.3/100) \cdot 1000 \cdot 170 \\ &= 510 \text{ mm}^2 \end{aligned}$$



Provide 10mm dia bars

$$\begin{aligned}\text{Spacing of 10mm dia bars} &= (1000 * 3.14 * (10^2/4)) / 894.6 \\ &= 87.99\text{mm}\end{aligned}$$

Provide 10mm dia bars @100 mm c/c Cantilever action

(BM) at 1H:

$$H^2/(D*t) \quad 1H$$

$$6 \quad -0.0187 \quad 7.8 \quad ?$$

$$8 \quad -0.0146$$

Interpolate:

$$\begin{aligned}(-0.0187 - (-0.0187 - (-0.0146)) * (7.8 - 6)) / (8 - 6) &= -0.015 \quad \text{Maximum cantilever moment} \\ = \text{Coefficient} * WH^3 & \\ &= -0.015 * 98109 * 4^3 \\ &= 9417.6 \text{ N.m}\end{aligned}$$

$$\begin{aligned}A_{st} &= BM / (\text{safe stress} * 0.85d) \quad (d = \text{eff. Cover of 30mm provide}) \\ &= 9417.6 * 10^3 / (150 * 0.85 * 140) \\ &= 527.6 \text{mm}^2\end{aligned}$$

$$\begin{aligned}A_{st \text{ min}} &= 0.3/100 * 1000 * 170 \\ &= 510 \text{mm}^2\end{aligned}$$

Provide 10mm dia bars

$$\begin{aligned}\text{Spacing} &= (1000 * 3.14 * (10^2/4)) / 527.6 \\ &= 148 \text{mm c/c}\end{aligned}$$

$$\begin{aligned}\text{Vertical steel distribution} &= 0.3\% \text{ gross are} \\ &= 510 \text{mm}^2\end{aligned}$$

Step 4: Base slab

Provide 150mm thick slab with top and bottom mesh with 10mm dia bars @ 250mm c/c.

## **RECTANGULAR WATER TANK RESTING ON GROUND**

Rectangular tanks are useful for smaller capacities and for large capacity circular water tank are preferred

.let centre line dimension of the tank be ( L\*1) where,

L= long plan dimension

L=short plan dimension

- i. If ( L/l <2) the wall is designed as continuous horizontal slab subjected to a water pressure 'Wh 'per unit area.
- ii. If ( L/l >=2) the long walls are considered as vertical continuous cantilevering for the whole pipe from base. The short walls are considered as spanning between long wall at end taken as fixed.

**PROBLEM 1** A reinforced concrete tank is 6m\*3m with a max depth of 2.5m of water. The tank rest on ground 150mm \*150mm. splays are provided at the junction of walls and base slab.

Design a tank using M<sub>20</sub> grade concrete and Fe 250 steel.

Solution:

Step1: given data

$$L=6\text{m}$$

$$L=3\text{m}$$

$$\text{Depth of water } =2.5\text{m}$$

150mm\*150mm splays are provided

$$M_{20} \text{ fe}415, L/l =6/3=2$$

Since the ratio is equal to 2, the long walls will be designed as vertical cantilever and short wall as spanning between the long walls

Step2: design of long wall

They are designed as vertical continuous since splays are provided effective height

$$\text{Effective ht} =2.5-0.15=2.35\text{m}$$

$$\text{Max BM per metre width of long wall} = (W*H^3)/6$$

Where,

W= specific wt

$$WH^3/6 = (9810*2.35^3)/6$$

$$= 21219 \text{ N.m}$$

This BM produces tension near water face

Moment of resistance (M.R) =  $Qbd^2$

In code book

M<sub>20</sub>: Fe 250; Q=1.33 &

M<sub>20</sub>: FE415; Q=1.16

Equate BM to M.R

$$21219 \times 10^3 = 1.33 \times 1000d^2$$

$$D = 126\text{mm} \quad (130\text{mm})$$

Provide an effective cover of 30mm

$$\text{Overall thickness of wall} = 130 + 30 = 160\text{mm}$$

$$\begin{aligned} A_{st} &= \text{BM} / (\text{safe stress} \times 0.85 \times d) \\ &= (21218 \times 10^3) / (115 \times 0.85 \times 130) \\ &= 1670\text{mm}^2 \end{aligned}$$

$$\begin{aligned} A_{st \text{ min}} &= 0.3\% GA \\ &= (0.3/100) \times 1000 \times 160 \\ &= 480\text{mm}^2 \end{aligned}$$

Provide 12mm dia bars

$$\begin{aligned} \text{Spacing of 12mm dia bars} &= (1000 \times 3.14 \times (12^2/4)) / 1670 \\ &= 68\text{mm} \end{aligned}$$

Provide 12mm dia bars @ 100 mm c/c

$$\begin{aligned} \text{Pull in long wall for bottom 1m ht} &= (\text{WHD})/2 \\ &= (9810 \times (2.35 - 1) \times 3) / 2 \\ &= 19865\text{N} \end{aligned}$$

$$\begin{aligned} A_{st} &= T / (\text{safe stress}) \\ &= 19865 / 115 \\ &= 172\text{mm}^2 \quad (\text{safe stress} = 115\text{N/mm}^2) \end{aligned}$$

$$A_{st \text{ min}} = 480\text{mm}^2$$

$$\begin{aligned} \text{Spacing of 12mm dia bar} &= (1000 \times 3.14 \times (12^2/4)) / 480 \\ &= 235\text{mm} \end{aligned}$$

Provide 12mm dia bars @ 100mm c/c.